



## Introduction of Nano & HP Fiber-Reinforced Link Slab Technologies

Celik Ozyildirim, Ph.D., P.E.

Date

**Name Surname**

Job Title, Division

*A partnership of the **Virginia Department of Transportation**  
and the **University of Virginia** since 1948*

# Outline

## Nanomaterials

- Titanium Dioxide
- Siliceous nano material (as SCM: supplementary cementitious material)
  - Nanosilica
  - Nanoclay

## Fiber reinforced concrete

- High-performance fiber reinforced cementitious composites (HPFRCC)
- Potential applications for HPFRCC (link slab, thin overlays)

# Titanium Dioxide – TiO<sub>2</sub>

- In almost every sunscreen
- Most widely used white pigment; used as a white food coloring; used to whiten skimmed milk; in paints
- In cosmetic and skin care products, used both as a pigment and a thickener
- In plastics and other applications for its UV resistant properties where it acts as a UV absorber

## IN CONCRETE

- Photocatalyst decompose organic materials and air-borne pollutants (self-cleaning concrete)

# SCM

- Fly ash – utilities
- Slag – iron making (not steel)
- Silica fume – ferro silicon
- Metakaolin – partially calcined clay
- Cement + Water  $\rightarrow$  C-S-H + CH      Hydraulic
- Pozzolan + CH + Water  $\rightarrow$  C-S-H      Pozzolanic

# Siliceous Nanomaterial

- Nanometer =  $10^{-9}$  meters
- Has particles with at least one dimension < 100 nm (average cement particle size is 15000 nm)
- Small particle size = more surface area per volume
- Types include nanoparticles, nanotubes,
- Very reactive, dense cement paste, improved paste aggregate interface
- Improvements in permeability, strength, shrinkage, ductility, and impact resistance

# VTRC Study (VTRC 10-R18)

- 26 Concretes
  - 2 Control Concretes
  - 3 Concretes with common SCMs
  - 21 Concretes with different types and amounts of nanomaterials (1 nanosilica [NS], 6 nanoclays [NC])
- Macroscale Testing (strength, E, permeability, length change)
- Microscale Testing (atomic force microscope [AFM], nanoindentation)

# SCM

<b>Designation</b>	<b>Replacement (%)</b>
Fly ash	21
Slag	40
Silica fume	7
NS	3
NC1	3
NC2	0.5
NC3	0.5, 2, 4
NC4	0.25, 0.5, 2
NC5	0.5, 1, 3
NC6	0.5, 1, 3

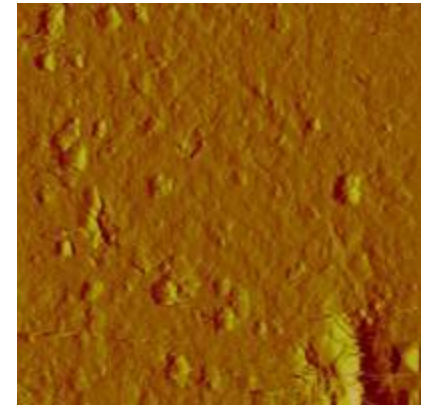
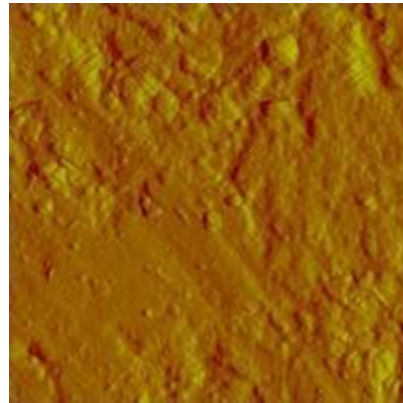
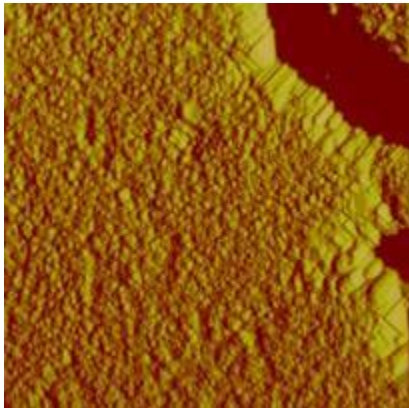
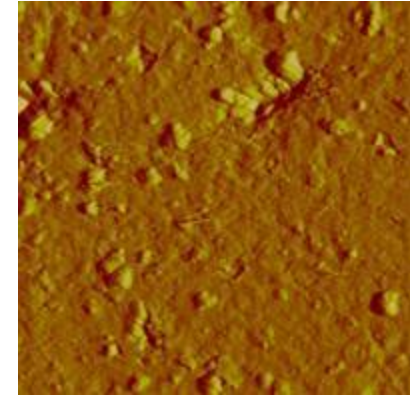
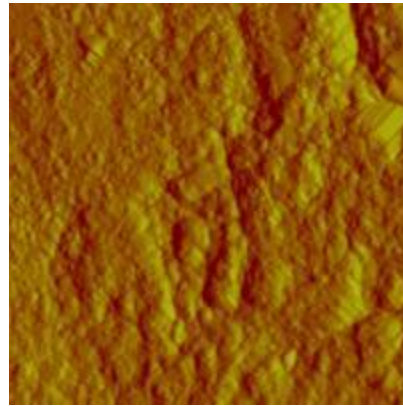
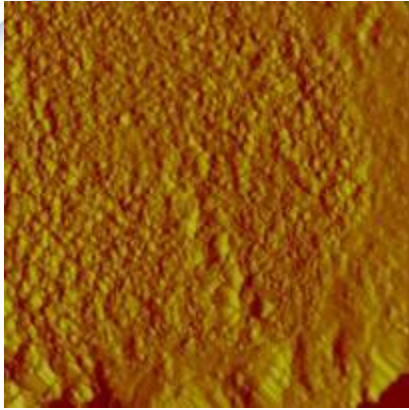
# Atomic Force Microscope - AFM

- An extremely fine sharp tip comes in contact or very close to the sample.
- Radius of curvature of the tip is on the order of nanometers.
- The sample is then scanned beneath the tip.
- Different forces either attract or repel the tip causing deflections.
- Deflections are measured using a laser and processed using imaging software.
- Image is a topographical representation of the sample.



# AFM Concrete Images

35.8  $\mu\text{m}$  by 35.8  $\mu\text{m}$



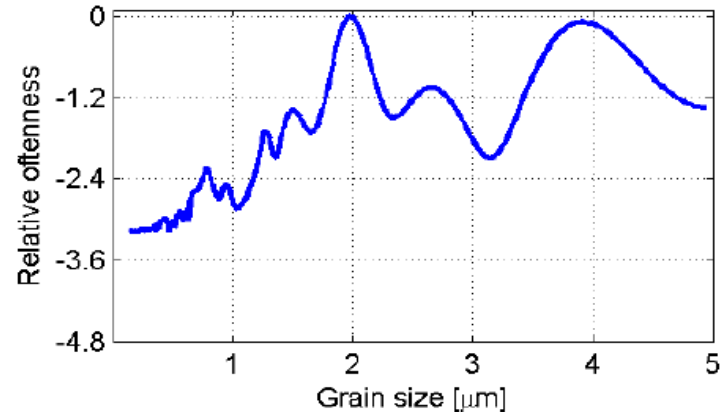
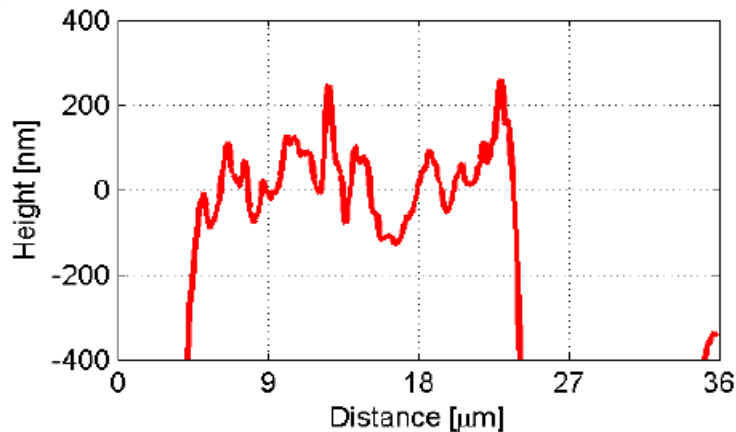
Regular

Silica Fume

Ludox

# AFM Image Analysis

- Power Spectral Density

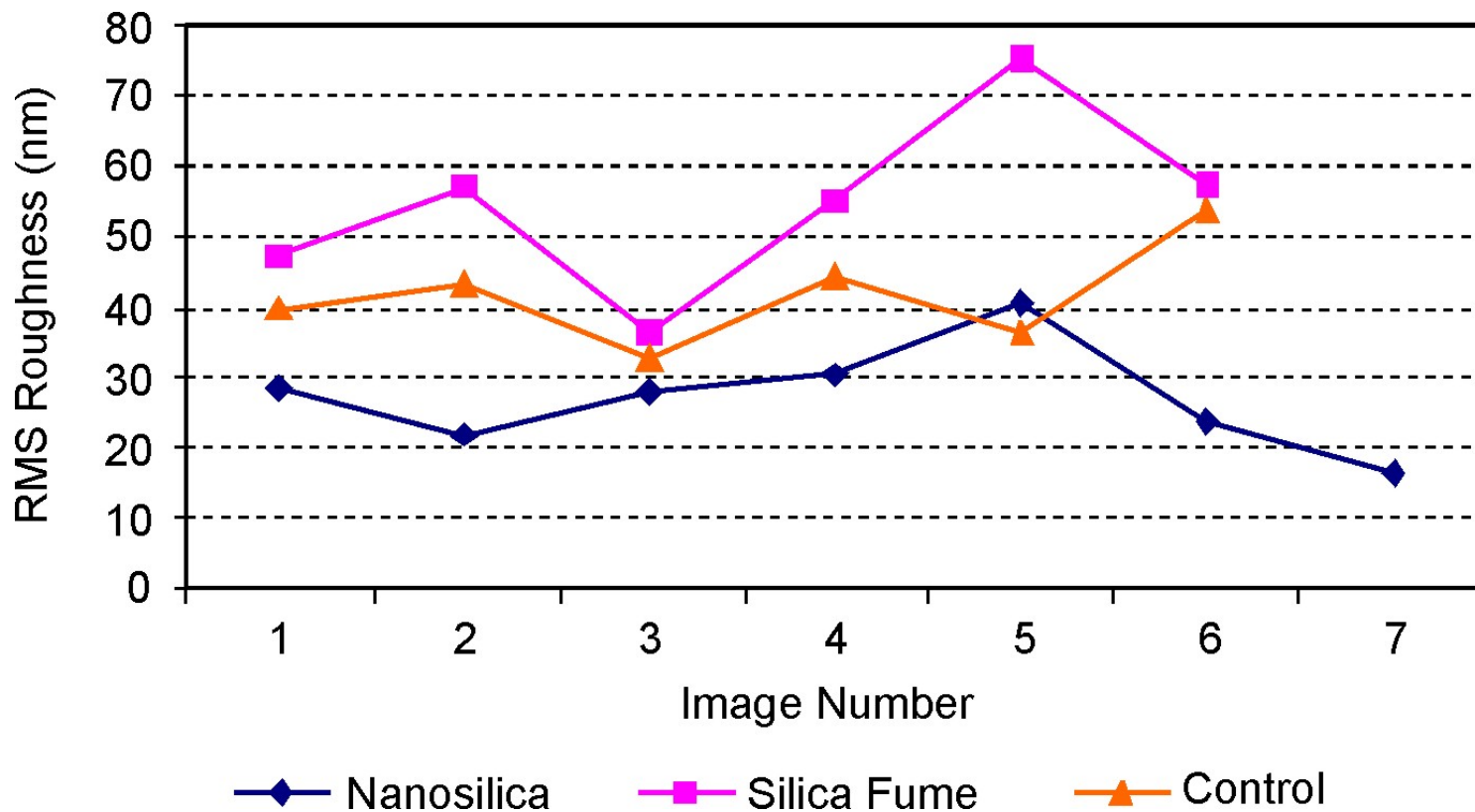


## Root mean square (RMS) Roughness

Integrate the PSD over a frequency and take the square root

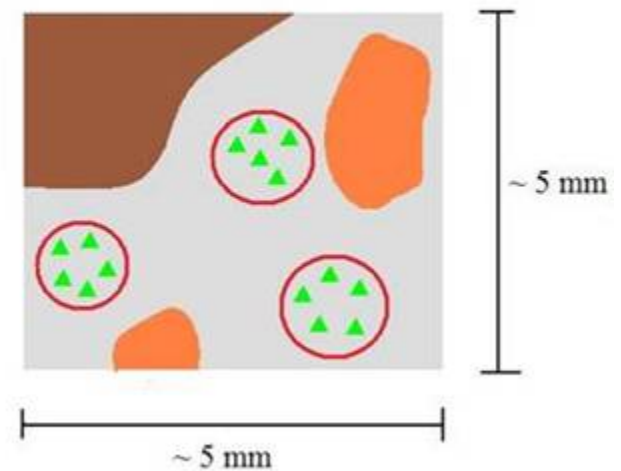
Roughness values indicate the uniformity of the paste

# Roughness Results



# Nanoindentation Details

- Indented using a Berkovich tip (3 sided pyramid)
- Indented 5  $\mu\text{m}$  into the surface



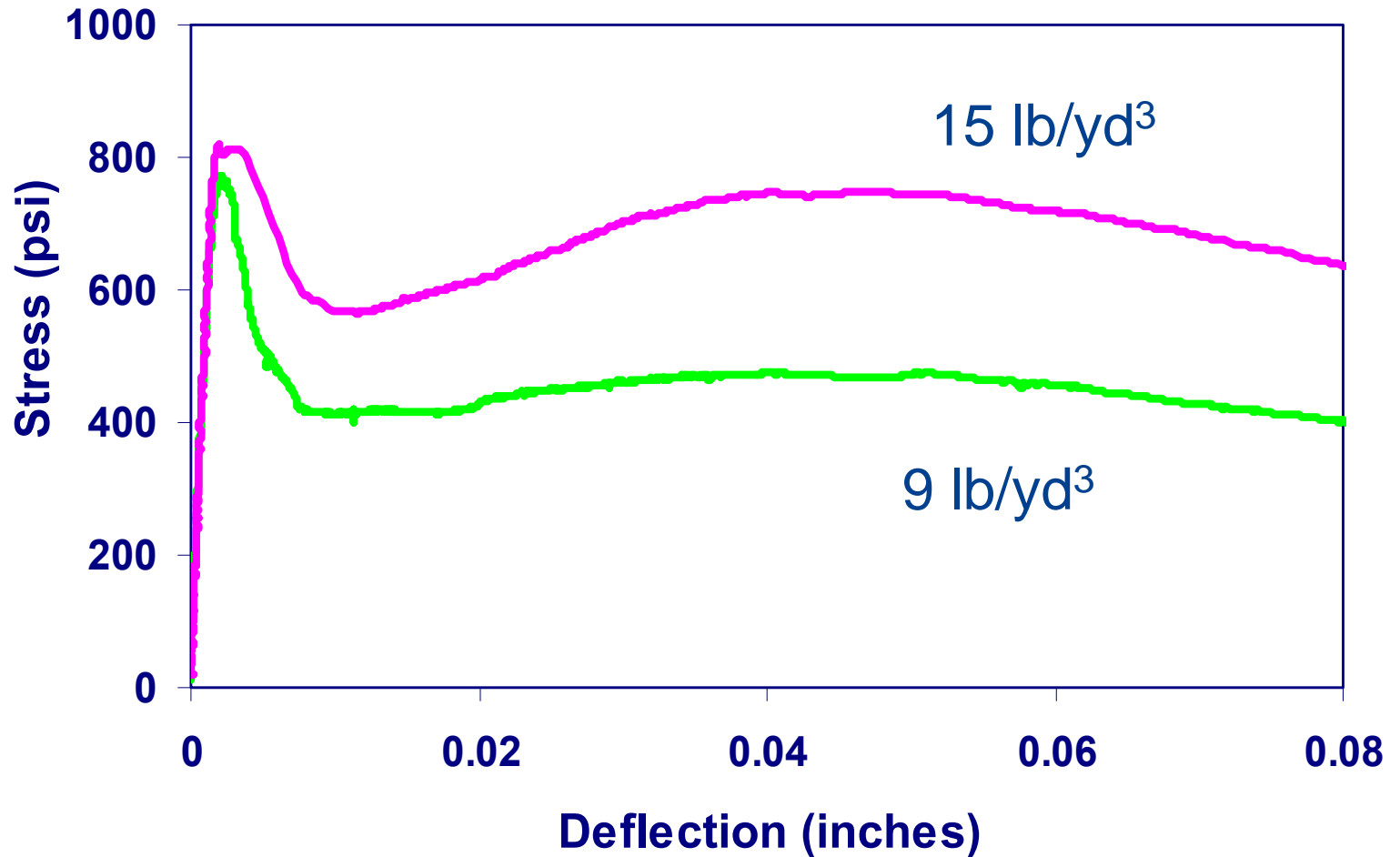
# Findings and Observations

- Difficult to obtain nanomaterials that can enhance concrete properties.
- Nanoparticles have the potential to improve properties at very small addition rates.
- Dispersion and clumping is an issue.
- Environmental impact due to size; powder versus solution

# Next Step

- Looking for nano particles
- Asking for effective dispersants
- Determine the optimum amounts

# Fiber Reinforced Concrete Residual Strength with Synthetic Fibers



# Route 11 over Maury River





# Route 106 over Chickahominy River



# Route 17 at Yorktown

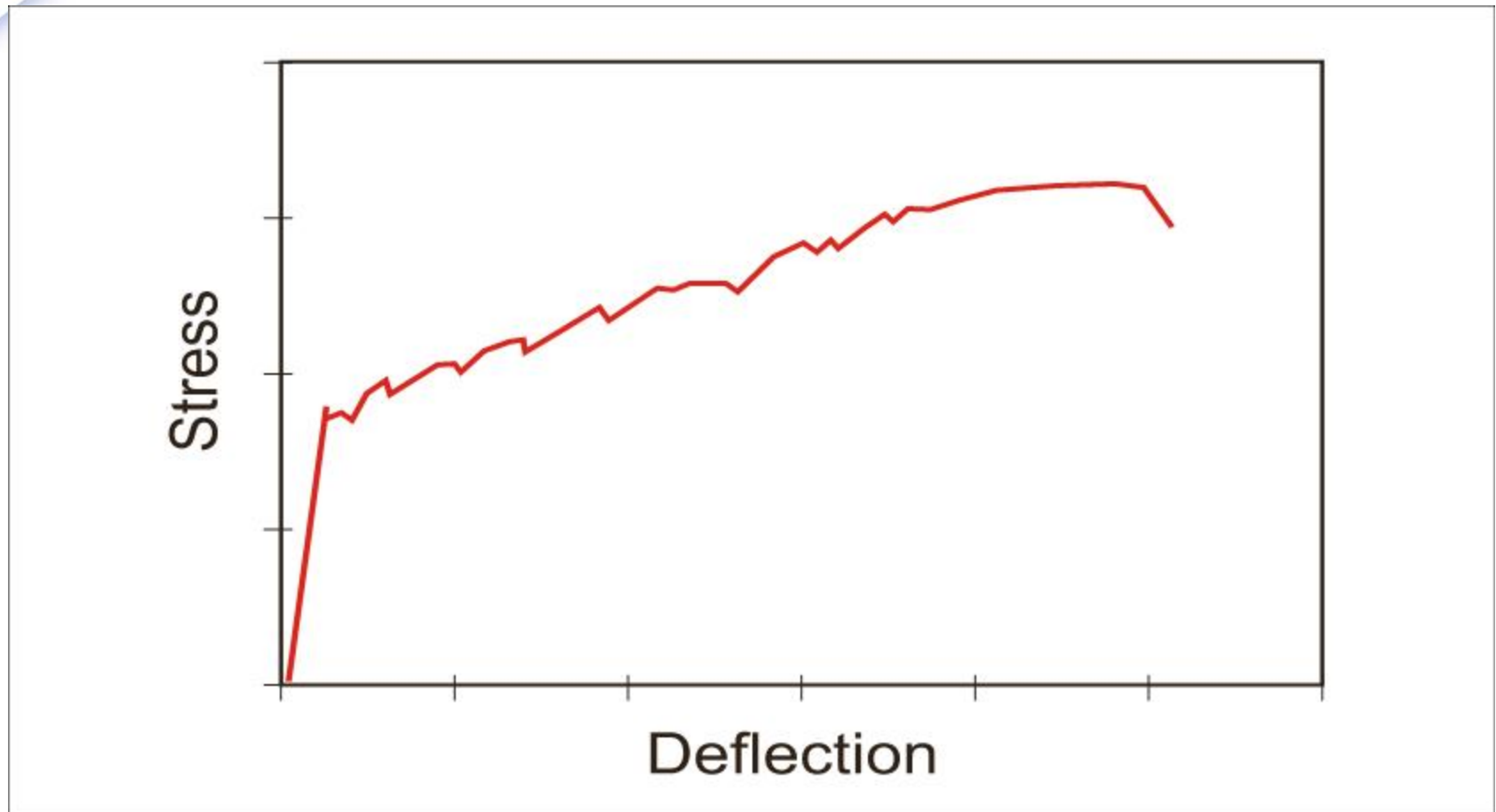


# FRC Crack Survey

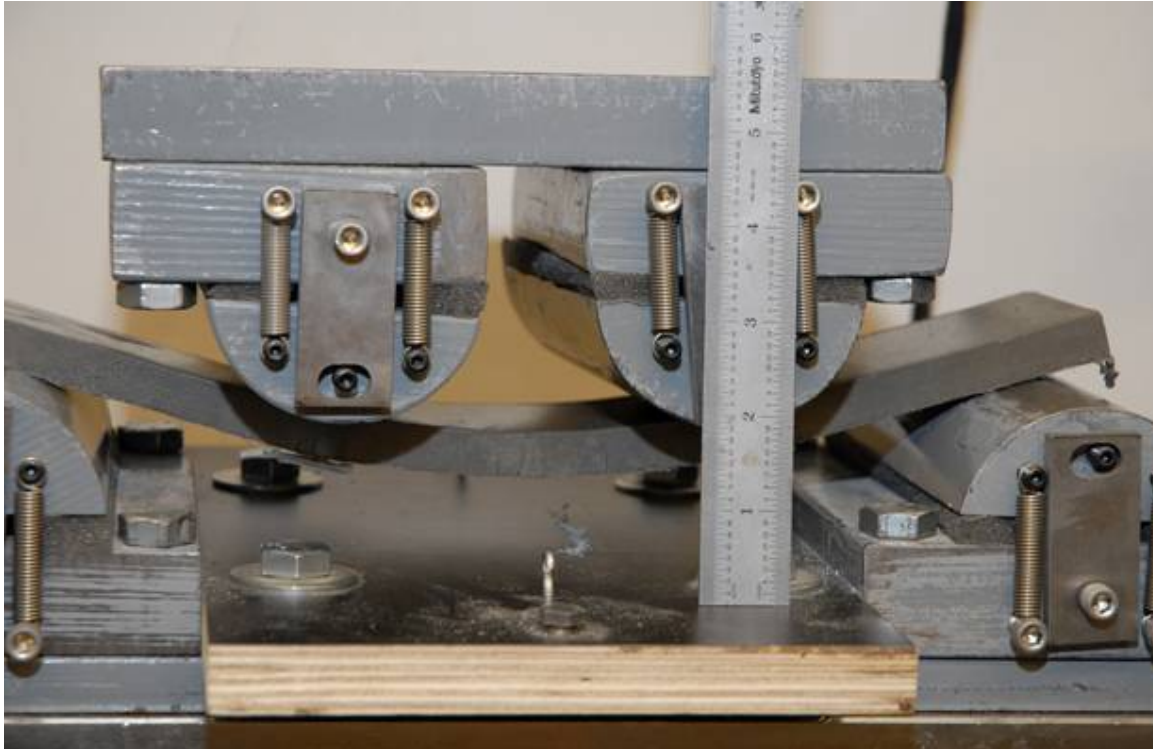
## Route 11, Lexington, VA - 5 years after

Dimension	Control	Fiber
Total Length (ft)	151	59
Average Width (mm)	0.53	0.29

# High Performance Fiber Reinforced Concrete Strain Hardening



# HPFRCC



# High Performance Fiber Reinforced Cementitious Composites - HPFRCC

- High ductility and toughness
- Strain hardening that leads to multiple micro-cracks at large deformations
- Maximum crack width  $<100 \mu\text{m}$ , even at ultimate load
- Increased durability and service life

# UHPFRC



# Engineered Cementitious Composite - ECC Bendable Concrete

- Dr. Victor Li, University of Michigan

## USED IN

- High rise buildings in Japan as coupling beams to mitigate earthquake damage
- Extruded pipes
- Repair of a dam and retaining wall in Japan
- Link slab in Michigan



# VTRC Study

(VTRC 08-R12)

- Type II Portland cement
- Class F fly ash
- Fibers
- Sand
- Water
- High-range water-reducing admixture

# Fibers

- PVA – Polyvinyl Alcohol
- Steel (SH) – Triangular cross-section and twisted along its length.
- Steel (SB) – Round in cross-section with hooked ends.

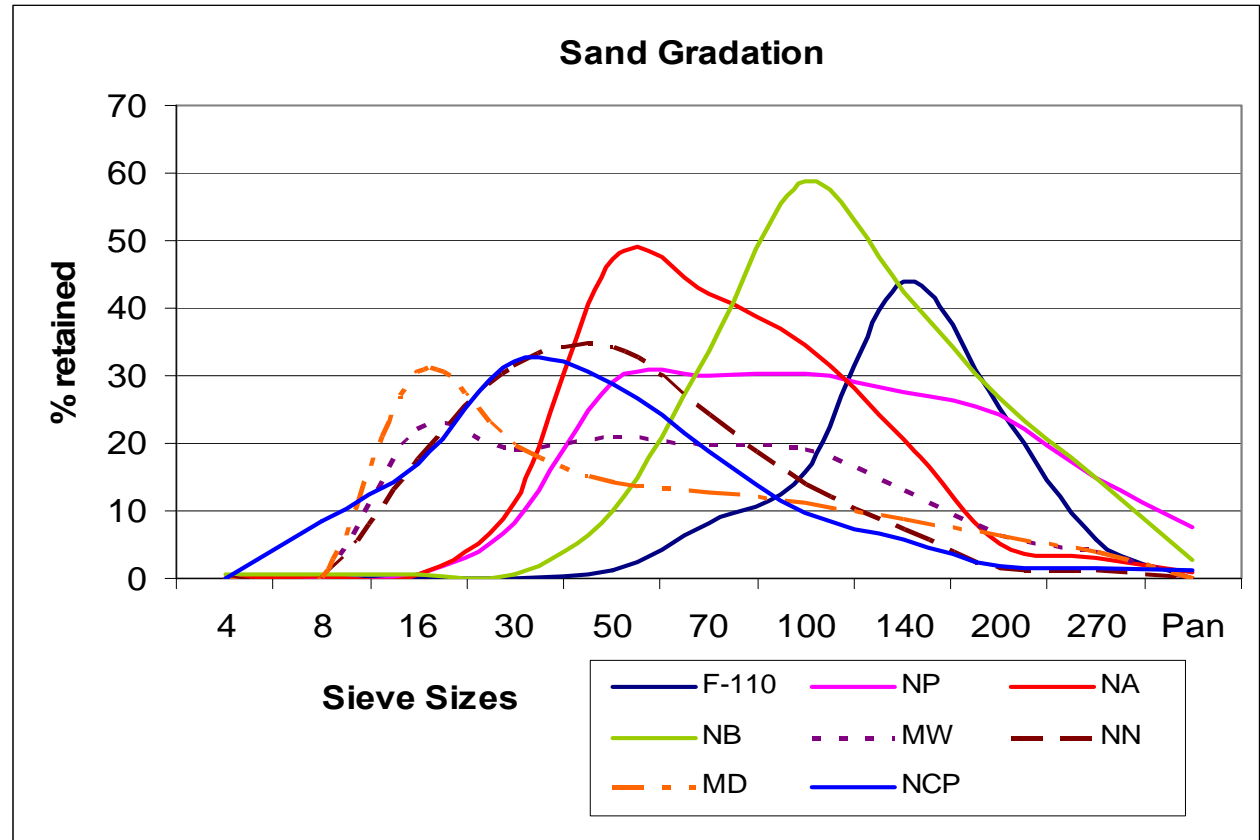


# VTRC Study - Sand

Special natural  
silica sand –  
F-110

Natural sand –  
(NA, NB, NP,  
NN, and NCP)

Manufactured  
sand – (MD and  
MW)



# Proportions

w/cm = 0.31

Fibers: 2% by volume

Fly ash/pc = 2.2

Ingredient	lb/yd <sup>3</sup>
Water	644
Cement (pc)	651
Fly Ash	1428
Sand	755
PVA fiber	44

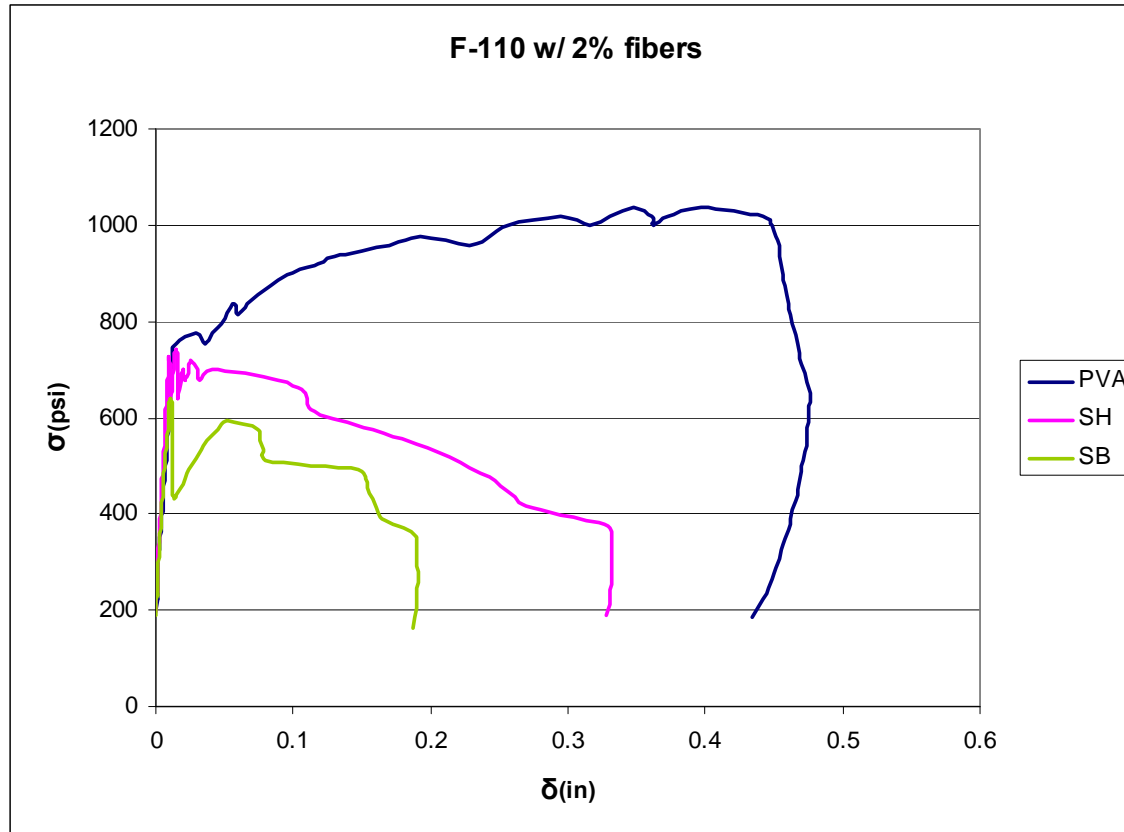
# Strength

- Cube strengths exceeding 3000 psi were obtained at 28-days.
- At 90-days, strengths exceeded 4000 psi with most exceeding 6000 psi.
- At 28-days, flexural strength of 1-in beams exceeded 600 psi except one.
- At 90-days, flexural strengths were high with many exceeding 1000 psi.

# Permeability, Shrinkage, and Freeze/Thaw Resistance

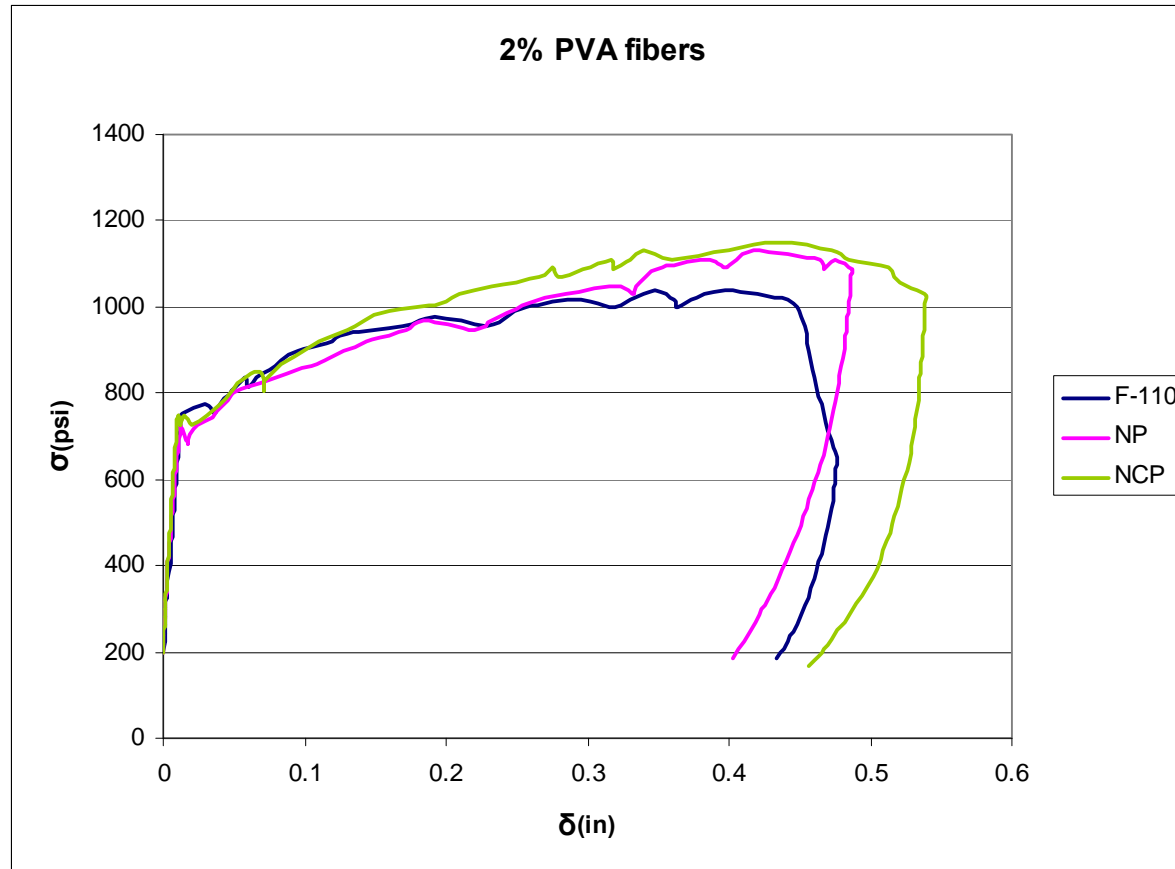
- Very low permeability
- High shrinkage (due to high cement content and low water cementitious material ratio) but expected to have very tight cracks
- Acceptable freeze/thaw resistance in laboratory tests; even though, and air entraining admixture was not added.

# Flexural Stress vs Midspan Deflection



1-in beams with PVA, SH and SB fibers mixed with F-110 sand at 28 days.

# Flexural Stress vs Midspan Deflection



1-in beams with F-110, mortar (NP) and concrete (NCP) sand and PVA fibers at 28 days.



## Deflection at Failure



Large deflections in beams with steel (SH) fibers (left) and PVA (right)

## Cracking Pattern at Failure



Cracks in beam with steel (SH) fibers (left) and PVA (right)

# Findings and Observations

- HPFRCC with PVA provided strain hardening and multiple micro-cracking using locally available sand.
- HPFRCC with PVA fibers can provide high residual strength up to 0.5-in deflection in 1-in thick beams. In 4-in beams the deflection was less, but still reached a large average value of 0.13-in.

# Findings and Observations

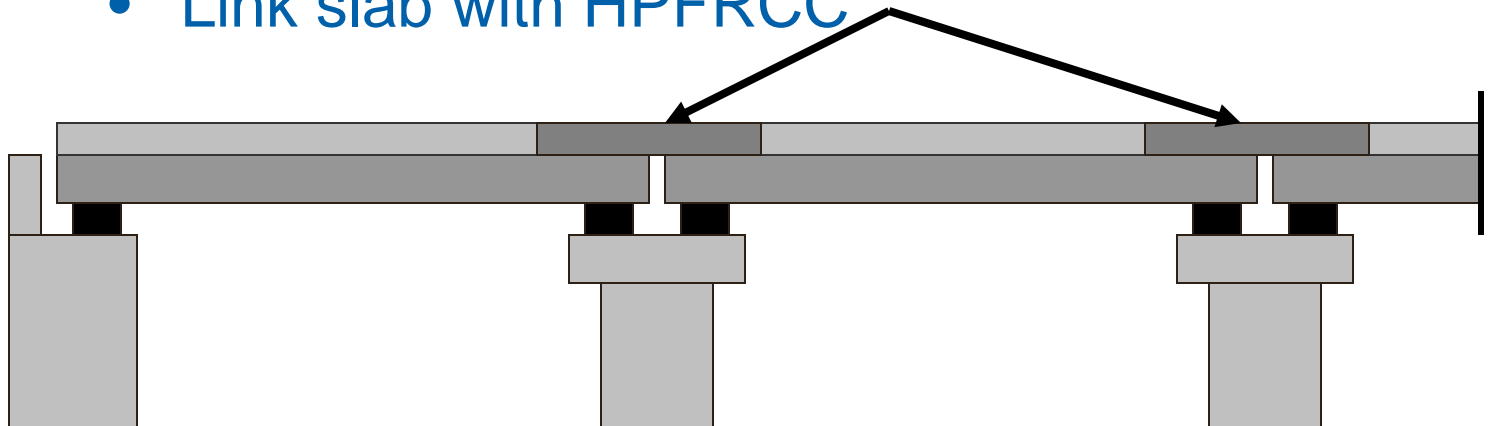
- Permeability of HPFRCC with high amount of Class F fly ash is very low.
- Shrinkage of HPFRCC is large; however, distress is not expected due to the presence of fibers that provide high tensile strain capacity.
- Non air-entrained HPFRCC can provide satisfactory freeze/thaw durability.
- Recommend trying in link slabs and thin overlays.

# Leaking Joints



# Link Slab

- Joints leak
- Eliminate joints
- Link slab with HPFRCC



# Michigan Study

- First ECC link slab completed in December, 2005
- Grove Street Bridge over I-94 in Ypsilanti
- 40 cubic yards of ECC in ready mixed concrete trucks
- A length of 7.5% of each adjacent bridge span was replaced with ECC to accommodate the thermal, shrinkage, and live load deformation
- 16'-11" in the longitudinal direction and 66'-5" in the transverse
- High skew angle of 45 degrees
- Early age cracking: crack widths, approximately 0.006" to 0.01", < AASHTO limit of 0.013", but > ECC of 0.004" (0.1 mm).
- The crack width did not increase after 3 winters.

# Michigan Study

Early Age Cracking – MDOT Report RC-1506, 2008

## Potential Sources of early age cracking

- Excessive shrinkage of ECC
- High restraint
- Epoxy reinforcement as stress concentrator: Nearly all cracks seem to initiate from steel reinforcement (may be due to change of geometry and weak bonding between ECC and ECR)
- Early age shrinkage before  $\sigma$ - $\delta$  curve development
- High skew angle: link slabs not to exceed  $25^\circ$
- Improper curing: wet burlap and plastics were not applied until several hours later, no continuous wet curing for the first several days



# Michigan Study

## New Link Slab Special Provision 1/2008

- Skew no more than 25°
- Expansive cement
- Shrinkage reducing admixture
- Max shrinkage at 28 days: 1 100 microstrain
- Max average crack width: 0.004 in (0.1 mm)
- Night placement during summer when the air temperature > 50 °F
- Curing compound upon finishing, wet curing as soon as surface supports for at least 7 days

# Next Step

- Place it in a link slab
- Place it in a thin overlay (1 inch or more)



**Thank You**

*A partnership of the **Virginia Department of Transportation**  
and the **University of Virginia** since 1948*